# MIX PROPORTIONING OF RECYCLED STEEL FIBER REINFORCED SELF COMPACTING CONCRETE

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ABSTRACT. In this paper it has been investigated that how the fresh and solidified properties of Self compacting concrete (SCC) influenced by recycled steel fiber (RSF) obtained from end life of used tires. The recycled steel fiber of length 35 mm and aspect ratio of 22 was used at percentage of 0.5, 1 and 1.5% by volume. The slump flow and J ring apparatus were primely used to check the workability criteria as per EFNARC 2005 guideline because yet there is no codal provisions or guidelines made for fiber reinforced self-compacting concrete. To make a sustainable and economically viable mix, mineral admixtures i.e. Fly ash and silica fume were used which also helps in increasing workability and viscosity respectively. Grading of aggregate was also done to get desire workability. Super plasticizer was used to increase flowability of mix. And seven days compressive strength checked to get the desire proportioning of the mix.

**Keywords:** SCC, Recycled steel fiber, Fly ash, workability, silica fume

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## INTRODUCTION

The waste management of used tires is major concern for many environmental bodies and agencies worldwide. The problem has drawn attention of planners, environmentalists, consumers and industry in the developed countries in Western Europe, USA, Japan, Australia etc. where billions of used tires are stockpiled. These stockpiles are also direct loss of energy and resources. Besides of it may prone to fire, health hazard and other environmental issues. To use waste material by material recovery from used tires is undertaken by utilizing either mechanical or thermal degradation processes. The former reduces tire to steel fibers and granulated rubber whereas the later process breaks down the tires into steel, char, liquids and gases [1]. Timely action regarding recycling of used tires is necessary in view of the problem of disposing of used tires. keeping in view the increasing cost of raw material, resource constraints and environmental problems, including fire and health hazards associated with the stockpiles of the used tires [2,3].

SCC is characterized by its ability to consolidate under its own weight without any means of compaction or vibration. SCC has the ability to spread smoothly in congested reinforced elements due to its flowability and use of small size aggregates [7-12]. SCC has been used in several projects, including residential buildings or large infrastructure for densely reinforced elements such as walls, load transfers floors, precast elements, and offshore structures and for many other [11,13-15].

Nowadays, it is well documented that the fibers added to concrete can substantially improve many of its engineering properties [7]. Introducing fibers into the concrete matrix can improve its properties, and enable the utilization of high strength concrete, while maintaining a ductile behavior. Steel or synthetic fibers help to improve various mechanical properties, fire resistance and reduce plastic shrinkage of SCC as well as to enhance the sustainability of a SCC matrix [16-21]. It has been observed that the performance of Self compacting fiber reinforced concrete (SCFRC) was much better than that of the corresponding Normal Vibrated Fiber Reinforced Concrete (NVFRC). Optimum fiber content was governed by the workability requirement of the concrete mixture. Information on the mix proportioning methods of SCC and SCFRC is also available [8,22-24]. There are numerous studies which have been done on the behaviour of Normal vibrated concrete incorporating recycled steel fiber obtained from the waste tires [25-38].

## RESEARCH SIGNIFICANCE

Very little information exists in literature on the mix proportioning and fresh properties of SCC with recycled steel fiber [39-41]. So, this study mainly focuses on the proportioning of ingredient of concrete and form SCC with RSF.

## **EXPERIMENTAL PROGRAM**

# **Materials**

• Fibers

Recycled steel fibers of aspect ratio of 22 and diameter 1.65 was used in this study which was prepared from the steel wire obtained from the end life of scrap trucks tire. These wires are extracted through shredding process but there is little amount of tire rubber remained which

was removed by peeling out it from board cutter and made bunch of cleaned surface wire so that it is easy to cut by the abrasive tool in required size of fibers. In the present study three fiber volume fractions i.e. 0.5, 1 and 1.5 % was used.

#### Cement

Ordinary Portland Cement (OPC) of 43 Grade confirming IS 8112:1989 was used.

#### Filler material

Fly ash and silica fume was used as a replacement of cement to increase the workability and viscosity for stability of the mix respectively. The fly ash was provided by Ambuja Cement Plant Ropar. And silica fume was procured from Astra Chemicals, Chennai.

## Aggregates

Crushed stone aggregates of size 12.5 mm or below will be used as coarse aggregates (CA) in three size gradations i.e. 12.5-10 mm, 10-6.3 mm and 6.3-4.75 mm in percentage of 33%, 42% and 25% of amount of CA for mix and locally available coarse sand will be used as fine aggregates confirming to IS 383-2016.

#### Admixtures

A polycarboxylic ether-based superplasticizer Master Glenium 51 was used in suitable dosages to obtain the required SCC and SCFRC mixes with different volume fractions of steel fiber.

### Methodology

The basic mix proportion for SCC was done on the basis of Nan Su method [23] and Modified Nan Su Method [24] as follows:-

1. Calculation of quantity of fine and coarse aggregate

The packing factor (PF) find out experimentally 1.10 by proportioning the gradation of coarse aggregates (density  $w_{ca} = 1504 \text{ kg/m}^3$ ) and fine aggregates (density  $w_{fa} = 1400 \text{ kg/m}^3$ ). The volume of FA to all in aggregate (s/a) ratio taken from the literature which ay

be 0.50-0.57 but in this study it was consider 0.59 due to fiber inclusion.

$$\begin{aligned} W_{fa} &= PF \times w_{fa} \times (s/a) \\ &= 1.1 \times 1400 \times 0.59 = 908.6 \text{ kg/m}^3 \end{aligned}$$

$$W_{ca} = PF \times w_{ca} \times (1-s/a)$$
  
= 1.1 \times 1504\times (1-0.59) = 678.3 kg/m<sup>3</sup>

# 2. Calculation of cement content

The design strength of concrete (f<sub>c</sub>) opted 40 MPa

$$C = 7f_c \times C.F. = 7 \times 40 \times 1.38 = 386.4 \text{ kg/m}^3$$

Where CF represent correction factor as per method proposed by [24].

3. Calculation of mixing water content required by cement

$$W_{wc} = (w/c) \times C = 0.58 \times 386.4 = 224.11 = 224.11 \text{ say } 224 \text{ kg/m}^3$$

4. Calculation of fly ash (FA) and silica fume (SF) contents

Total volume of filler  $V_{pf}$  for the assumed air entrained  $V_a$ = 1.5%

$$\begin{split} V_{pf} &= 1 \text{--} \left(W_{ca} / \left(1000 \times G_{ca}\right)\right) \text{--} \left(W_{w} / \left(1000 \times G_{w}\right)\right) \text{--} \left(W_{fa} / \left(1000 \times G_{fa}\right)\right) \text{--} \left(C / \left(1000 \times G_{c}\right)\right) \text{--} V_{a} \\ &= 1 \text{--} \left(678 / \left(1000 \times 2.65\right)\right) \text{--} \left(224 / \left(1000 \times 1\right)\right) \text{--} \left(909 / \left(1000 \times 2.64\right)\right) \text{--} \left(386 / \left(1000 \times 3.15\right)\right) \\ &\quad - 0.015 \\ &= 0.0383 \text{ m}^{3} \end{split}$$

Now the amount of filler calculated as follows

$$\begin{split} V_{pf} &= (1+\text{w/FA}) \times \text{A\%} \times (W_{pm}/1000 \times G_{fa}) + (1+\text{w/SF}) \times \text{B\%} \times (W_{pm}/1000 \times G_{sf}) \\ 0.0383 &= ((1+0.58) \times 0.6 \times W_{pm}/1000 \times 2.10) + ((1+0.58) \times 0.4 \times W_{pm}/1000 \times 2) \\ W_{pm} &= 49.93 \text{ say } 50 \text{ kg/m}^3 \end{split}$$

Now, content of Fly Ash = 
$$W_{pm} \times A\% = 50 \times 0.6 = 30 \text{ kg/m}^3$$
  
content of Silica Fume =  $W_{pm} \times B\% = 50 \times 0.4 = 20 \text{ kg/m}^3$ 

- 5. Calculation of mixing water content for fly ash and silica fume needed in SCC Water required for Fly Ash  $(W_{FA})=30\times0.58=17.4~kg/m^3$  Water required for Silica Fume  $(W_{SF})=20\times0.58=11.6~kg/m^3$
- 6. Calculation of SP dosage

$$\begin{split} W_{SP} &= 0.60 \times (386 + 30 + 20) \ / \ 100 = 2.62 \ kg/m^3 \\ Amount of water in SP \\ W_{SP} &= (1\text{-}0.4) \times 2.62 = 1.57 \ kg/m^3 \end{split}$$

7. Adjustment of mixing water content needed in SCC

$$W = 224 + 17.4 + 11.6 - 1.57 = 251.43 \text{ kg/m}^3$$

8. Trial mixes and tests on SCC properties trials were made to obtain suitable SCC (table 1)

Table 1 Mix for SCC after trials

CEMENT	FLY	SILICA	FINE	C.A.	WATER	SP
	ASH	<b>FUME</b>	<b>AGGREGATES</b>			
386	78	52	817	503	251	3.61

## 9. Adjustment of mix proportions

There were adjustments made in amount of water, superplasticizer, FA and SF for maintaining workability and viscosity in the mix respectively. Also, it helps to make mixture more flowable and stable without adding viscosity modifying agent (VMA) requirement. Haddadou 2014 [42] and [43], investigate that increased amount of binders and altering in fine aggregates provide better dispersion of fibers and compensate for the effect of randomly distribution of fibers which causes porosity [43].

Now the obtained mix proportion from the above method as follows (table 2): -

Table 2- Mix after adjustment

CEMENT	FLYASH	S.F.	F.A.	C.A.	WATER	SP (%)
$(kg/m^3)$	$(kg/m^3)$	$(kg/m^3)$	$(kg/m^3)$	$(kg/m^3)$	$(kg/m^3)$	
389.16	376.65	92.94	812.50	500.11	229.78	0.7%

#### 10. Inclusion of fibers in SCC

As Grunewald and Wal raven 2001 [43] investigate that fibers have a long shape and compare with the aggregate of the same volume, a higher specific surface. Zhao and Du 2008[44] replace the equal amount of coarse aggregate with equal amount of steel fiber. And the quantity of steel fiber is consider in the sand ratio but this technique was used for normal vibrated concrete. In this study the paste and fine aggregate amount is more aggregate content. So here the coarse aggregate was directly replaced by equal amount of weight of the fiber content. Proceeding this approach, the mixes were formed and amount of SP also adjusted as per requirement of mix based on trials as tabulated in table 3-

Table.3 final mix proportion with and without fibers

Mix ID	C (kg/m <sup>3</sup> )	FyA (kg/m <sup>3</sup> )	S.F. (kg/m <sup>3</sup> )	F.A. (kg/m <sup>3</sup> )	C.A. (kg/m <sup>3</sup> )	Water (kg/m <sup>3</sup> )	SP (%of TOTAL BINDER	FIBER CONTENT (kg/m³)
							CONTENT)	
M0	389.16	376.65	92.94	812.50	500.11	229.78	0.7%	_
M1	389.16	376.65	92.94	812.50	460.86	229.78	0.8%	39.25
M2	389.16	376.65	92.94	812.50	421.61	229.78	0.8%	78.50
M3	389.16	376.65	92.94	812.50	382.36	229.78	0.8%	117.75

# Mixing procedure

For mixing a drum mixture was used. Firstly, fine and coarse aggregate were dry mixed for 3 min. In this amount of water was also added as per water absorption of aggregate to made them surface saturated condition. Then Fly ash, silica fume and cement were added and mixed the whole quantity for 5 min. Now, the water with required amount of superplasticizer added up to 90% of total amount of water and SP, and mixed for 5 mins. Add fiber by sprinkling by hand and remaining 10 % water added and mix it for 3 to 5 min till the sign of forming SCC.

## **RESULT AND DISCUSSION**

# Fresh Properties and compressive strength

There is no codal criteria to check properties of steel fiber reinforced self- compacting concrete (SFRSCC) yet, so to check the fresh properties of SCC with fiber EFNARC 2005 for SCC was used. For the preliminary examination for SFRSCC here only two criteria of flowability was checked one slump flow test and another one J ring test. The result obtained is given in table 4 from the result it was observed that the flowability is reduced as the fiber volume was increased as shown in **Fig. 1** (a) and (b), the same result was shown by Matsali et. Al (2017,2016) [39,41]. There is increase in compressive strength as the fiber content

increased as show in table 4. the same pattern was show in the study by Matsali et. Al (2017) [41].

Table .4	Result of	Slump flow.	J Ring test	and 7 day	compressive	strength of mixes

MIX ID	SLUMP FLOW (MM)	J RING TEST (MM)	COMPRESSIVE STRENGTH (7 DAYS IN MPA)
M0	760 mm	730	26.05
M1	720 mm	700	28.45
M2	710 mm	685	29.80
M3	705 mm	670	31.70





Figure 1 (a) J ring for SCC with No fiber content (M0) and (b) J ring for SCC with 1.5 % fiber content (M3)

# **CONCLUSION**

On the basis of experimental work, the following finding was observed-

- On the basis of J ring Test and slump flow test it can be stated that the workability was affected by the inclusion of RSF in concrete and the superplasticizer demand increases.
- The compressive strength was increased with increasing the RSF in the mix.

The RSF increase its strength by incorporating in concrete. So, it is beneficial to use RSF as reinforcement in SCC.

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